

**WILBUR D. THESIER, PE PC
PROFESSIONAL ENGINEERS**

**P.O. BOX 870
CARTHAGE, NY 13619
PHONE 315 493-1966
FAX 315 493-0541
email: wthesier@verizon.net
solmste@twcny.rr.com**

September 18, 2012

Kurt W. Hauk, P.E.
City Engineer
Room 305, City Hall
245 Washington Street
Watertown, NY 13601

Re: Site Plan Submission
Parkside Bible Church Addition
491 Eastern Boulevard

Dear Mr. Hauk:

Attached to this submission please find the following:

- 3 sets, including:
 - Cover letter
 - Engineering report
 - Application form
 - Full size drawings (one set stamped)
- 13 sets, including:
 - Cover letter
 - Application form
 - Reduced size drawings

A pdf file of the submission will also be provided via email to your office. If there are any questions, please feel free to contact me.

Sincerely,



Steven J. Olmstead, P.E.

cc: Attachments



**CITY OF WATERTOWN
SITE PLAN APPLICATION
AND
SHORT ENVIRONMENTAL
ASSESSMENT FORM, PART 1**

** Provide responses for all sections. INCOMPLETE APPLICATIONS WILL NOT BE PROCESSED. Failure to submit required information by the submittal deadline will result in **not** making the agenda for the upcoming Planning Board meeting.

PROPERTY LOCATION

Proposed Project Name: New Addition to Parkside Bible Church
Tax Parcel Number: 5-26-103.007
Property Address: 491 Eastern Boulevard
Existing Zoning Classification: LI

OWNER OF PROPERTY

Name: Parkside Bible Church of C&ME
Address: 491 Eastern Boulevard
Watertown, NY 13601
Telephone Number: 315-782-6534
Fax Number: _____

APPLICANT

Name: Lundy Group of Companies
Address: 35794 NYS Rt. 126
Carthage, NY 13619
Telephone Number: 315-493-2493
Fax Number: 315-493-2004
Email Address: frontdesk@mlundygroup.com

ENGINEER/ARCHITECT/SURVEYOR

Name: Wilbur D. Thesier, PE PC (Steven J. Olmstead, PE)
Address: PO Box 870
Carthage, NY 13619
Telephone Number: 315-493-1966
Fax Number: 315-493-0541
Email Address: solmste@twcny.rr.com

PROJECT DESCRIPTION

Describe project and proposed use briefly:

Project will include construction of an approximately 12,500 sf addition to the existing building, new parking, site lighting, landscaping and utility connections.

Is proposed Action:

New Expansion Modification/Alteration

Amount of Land Affected:

Initially: 8.4 Acres Ultimately: 8.4 Acres

Will proposed action comply with existing zoning or other existing land use restrictions?

Yes No If no, describe briefly

What is present land use in vicinity of project?

Residential Industrial Commercial Agriculture
 Park/Forest/Open Space Other

Describe: _____

Does project involve a permit approval, or funding, now or ultimately from any other Governmental Agency (Federal, State or Local)?

Yes No If yes, list agency(s) and permit/approval(s)
City of Watertown - Site Plan Approval

Does any aspect of the project have a currently valid permit or approval?

Yes No If yes, list agency(s) and permit/approval(s)
Existing has previous site plan approval.

As a result of proposed project, will existing permit/approval require modification?

Yes No

Proposed number of housing units (if applicable): NA

Proposed building area: 1st Floor 12,500 Sq. Ft.
2nd Floor _____ Sq. Ft.
3rd Floor _____ Sq. Ft.
Total _____ Sq. Ft.

Area of building to be used for the boiler room, heat facilities, utility facilities
and storage: 400 Sq. Ft.

Number of parking spaces proposed: 14 new (112 existing)

Construction Schedule: April 2013 - August 2013

Hours of Operation: Mon - Fri 8 AM - 9 PM; Sat - Sun 8 AM - 4 PM

Volume of traffic to be generated: Mon-Sat 25 car trips/day;
Sunday 70 - 100 car trips/day. ADT

SIGNATURE

I certify that the information provided above is true to the best of my knowledge.

Applicant (please print) Michael Lundy

Applicant Signature _____ Date: _____

**WILBUR D. THESIER, PE PC
PROFESSIONAL ENGINEERS**

P.O. BOX 870

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**ENGINEERING REPORT
FOR
NEW ADDITION TO PARKSIDE BIBLE CHURCH
491 EASTERN BOULEVARD
WATERTOWN, NY**

SEPTEMBER 17, 2012

Project Description

The proposed project consists of an approximately 12,500 sf addition to the existing building at 491 Eastern Boulevard, as shown on the site plan (Sheet C101). The exterior elevations show that the addition will match the existing building in height and style. Detailed plans for the building construction are being developed, and will be submitted separately to City Codes Enforcement. The addition will be mixed use (gymnasium and classrooms). Two existing sheds on the property will be relocated, and some trees will be removed in the area of the proposed addition.

Parking

There are 112 existing parking spaces located to the south of the existing building, and 14 new spaces are proposed along the west side of the proposed addition. The existing parking spaces for the church are used on Sunday, while the new addition will be used Monday through Saturday; therefore the existing parking spaces will be utilized along with the proposed spaces. The proposed addition is to provide more space for the existing parishioners, and is not expected to increase the number of people at the facility.

Water and Wastewater

Additional water and wastewater flows are estimated at 800 gpd.

It is anticipated that sprinklers will be installed, and a 6" water service is proposed from the existing 16" steel water main on Huntington Street.

The existing facility is served by an on-site septic system. It is proposed that the addition and the existing building will be connected to the City sanitary sewer system. A 6" sewer main will be extended from the addition to a new manhole on the church property, then across Huntington Street to an existing manhole.

Storm Water

See separate drainage report.

Landscaping

New landscaping will be as shown on the drawings, and will match the existing landscaping.

Traffic

The number of vehicle trips per day from the existing facility is estimated at up to 25 per day on Monday through Saturday. On Sunday up to 200 people may attend services, and assuming 2 - 3 people per day results in 70 to 100 car trips per Sunday. As the addition is not expected to increase the number of people using the facility, traffic is not expected to increase.

Lighting

New lighting will be as shown on the drawings, and will consist of 2 new pole mounted lights and lighting on the building. Photometrics are also provided on the drawings.

ENGINEERING REPORT

STORMWATER HYDROLOGIC CALCULATIONS

FOR THE

PROPOSED MULTI-USE ADDITION

BY

PARKSIDE BIBLE CHURCH

491 EASTERN BOULEVARD/NYS ROUTE 3
CITY OF WATERTOWN,
JEFFERSON COUNTY, NEW YORK

SEPTEMBER, 2012

Prepared by:

WILBUR D. THESIER P.E., P.C.
P. O. Box 870
Carthage, New York 13619

1.0 NARRATIVE

1.1 GENERAL

The Parkside Bible Church has proposed the construction of an approximately 12,500± sq ft multi-use building Addition and related work, at their existing facility on an 8.376± acre parcel located at 491 Eastern Boulevard (T.M.P. # 5-26-103.007) in the City of Watertown, Jefferson County. A Location Map of the project site is attached.

The project site is bounded on the southeast by Eastern Boulevard/NYS Route 3, on the northwest by a residential apartment complex, and on the northeast by Huntington Street.

The proposed capital improvements at the site will increase the area of impervious surfaces and potentially increase stormwater runoff. The Applicant has proposed a wet bottom stormwater retention basin be provided to address water quality and quantity requirements to within the criteria established by the reviewing agencies.

1.2 PURPOSE

This hydrologic analysis assembles basic data and assumptions, and presents calculations regarding stormwater runoff from the project site and addresses the need to manage that runoff to meet the quantity requirements established by the reviewing agencies.

1.3 PROPOSED IMPROVEMENTS

The project site is currently used by the Parkside Bible Church as a church. The parcel has been partially developed with a building, parking for approximately 112 cars, two driveways to Eastern Boulevard, and supporting appurtenances. Except for a small wooded area on the northerly side of the site, the remainder of the parcel is a mowed, grassed lawn. The site has access to municipal water and sanitary sewer systems.

Capital improvements associated with the proposed development include: the construction of a 12,500 sf multi-use Addition; an extension of the internal asphalt road network and walks; parking for an additional 14 cars; grading; drainage and stormwater management facilities; and appurtenant work. The remainder of the disturbed work area will be topsoiled and seeded.

The proposed work is shown on Sheet C101 of the drawings.

2.0 ON-SITE STORMWATER MANAGEMENT HYDROLOGIC CALCULATIONS

2.1 GENERAL

This hydrologic analysis assembles basic data and assumptions, and presents calculations regarding stormwater runoff from the project site and addresses the need to manage that runoff to meet the quantity requirements established by the reviewing agencies. For the purposes of this analysis, reviewing agency requirements are interpreted as being post-development peak rates of stormwater runoff will be equal to or less than existing peak rates for the 1, 10 and 100 year storms. It is further proposed the retention basin contain sufficient volume to retain all runoff from the 1 and 10 year storms, and release less than 1 cfs for the 100 year storm

The parcel and existing building is located at the crest of a crowned area. Drainage from the site is primarily radially outwards from the building. Stormwater runoff from the southerly side of the existing building and a portion of the existing parking area sheet drains towards the south to Eastern Blvd/NYS Route 3. Similarly, runoff from the easterly side of the existing building and a portion of the existing parking area sheet drains towards the east to Huntington Street. Huntington Street lacks drainage facilities, including both roadside ditches and storm sewers.

It is proposed grading and drainage swales servicing the proposed capital improvements collect and convey runoff towards the northerly portion of the site, which is currently un-developed. No additional runoff will be directly towards Eastern Blvd/NYS Route 3. Runoff to Eastern Blvd is expected to decrease slightly due to a reduction of the tributary area.

It is proposed a wet bottom stormwater retention basin be provided to address water quality and quantity requirements. It is further proposed the retention basin be located in the northerly corner of the parcel. Outflow from the stormwater management basin would discharge to a swale and northerly towards Huntington Street.

2.2 STORMWATER RUNOFF PARAMETERS

As mentioned in Section 2.1, grading and drainage swales servicing the proposed capital improvements will collect and convey runoff towards the northerly portion of the site and into a proposed retention basin. Runoff from areas on the southerly and easterly sides of the parcel not impacted by the proposed improvements will be unchanged and will be excluded from further analysis. Only the central and northerly portions of the parcel, together with the areas draining into the proposed collection swales and retention basin, containing approximately 2.78± acres, will comprise the study area and be analyzed further.

Soils at the project site have been classified by the USDA Natural Resources Conservation Service as “Collamer silt loam” soils. These soils are further classified as belonging to Hydrological Soil Group C.

Peak rates of stormwater runoff from the drainage study area was computed using the USDA Natural Resources Conservation Service method. Weighted curve numbers (CN) for the drainage

study area was computed in Appendix A and summarized as follows:

Existing: 74.5
 Post-development: 80.6

The drainage basin's times of concentration were also computed using the USDA Natural Resources Conservation Service method. Calculations are computed in Appendix A and summarized as follows:

Existing: .43 hr
 Post-development: .40 hr

Peak rates of stormwater runoff were calculated for the drainage study area for the following storm frequencies and 24-hour rainfall amounts:

1 yr 2.2"
 10 yr 3.5"
 100 yr 4.75"

2.3 STORMWATER MANAGEMENT BASIN

As mentioned in Section 2.1, it is proposed a wet bottom stormwater retention basin be provided to address water quality and quantity requirements. It is further proposed the retention basin be located in the northerly corner of the parcel. It is further proposed the retention basin contain sufficient volume to retain all runoff from the 1 and 10 year storms, and release less than 1 cfs for the 100 year storm.

Features of the proposed stormwater management basin include the following:

- Bottom elevation = 93.0
- Outlet Control Structure: none
- A grassed emergency spillway/overflow will be set at 96.5'
- Top of the 10 ft wide embankment will be set at 97.5'

The stage-storage relationship for the basin is as follows:

<u>Contour Elevation</u>	<u>Area (sf)</u>	<u>Ave. Area (sf)</u>	<u>Incre. Height</u>	<u>Incre Volume</u>	<u>Summed Volume (cf)</u>
97.0	13,416				
96.75	12,795	12,484	0.25	3,121	28,534
96.5	12,174	11,553	0.5	5,776	25,413
96.0	10,932	9,754	1	9,754	19,637
95.0	8,576	6,538	1	6,538	9,883
94.0	4,500	3,345	1	3,345	3,345
93.0	2,190				

2.4 MANAGEMENT OF 1 YEAR STORM EVENT

2.4.1 Existing / Pre-Development Site Stormwater Runoff

The pre-developed site's 1-year storm's runoff was generated using the US Army Corps of Engineer's HEC-1 computer model. Pre-Development stormwater runoff calculations are summarized in Appendix A.

Peak Flow (1 yr storm, pre-development cond.) = 1.03 cfs

2.4.2 Developed Site Stormwater Runoff

It is proposed all runoff be retained for this storm event. No secondary outlet has been provided.

The developed site's 1-year storm's runoff was generated and routed through the stormwater management basin using the US Army Corps of Engineer's HEC-1 computer model. The basin's stage-storage relationship is presented in Article 2.3. For the proposed stage-storage-discharge relationship, the model computed the following values:

Peak basin inflow: 1.89 cfs

Max. water surface in the basin: 94.50'

Peak outflow rate: 0.0 cfs. (some exfiltration has been assumed)

A copy of the computer model's output is attached in Appendix A.

2.4.3 Conclusions

The calculations indicate that for the 1-year storm, outflow from the proposed basin will be 0.0 cfs, less than the pre-development peak flow rate of approximately 1.03 cfs. **(OK)**

Maximum water surface in the basin will be approximately 94.50'. **(OK)**

2.5 MANAGEMENT OF 10 YEAR STORM EVENT

2.5.1 Existing / Pre-Development Site Stormwater Runoff

The pre-developed site's 10-year storm's runoff was generated using the US Army Corps of Engineer's HEC-1 computer model. Pre-Development stormwater runoff calculations are summarized in Appendix A.

Peak Flow (10 yr storm, pre-development cond.) = 3.28 cfs

2.5.2 Developed Site Stormwater Runoff

It is proposed all runoff be retained for this storm event. No secondary outlet has been provided.

The developed site's 10-year storm's runoff was generated and routed through the stormwater management basin using the US Army Corps of Engineer's HEC-1 computer

model. The basin's stage-storage relationship is presented in Article 2.3. For the proposed stage-storage-discharge relationship, the model computed the following values:

Peak basin inflow: 4.67 cfs

Max. water surface in the basin: 95.42'

Peak outflow rate: 0.0 cfs. (some exfiltration has been assumed)

A copy of the computer model's output is attached in Appendix A.

2.5.3 Conclusions

The calculations indicate that for the 10-year storm, outflow from the proposed basin will be approximately 0.0 cfs, less than the pre-development peak flow rate of approximately 3.28 cfs. **(OK)**

Maximum water surface in the basin will be approximately 95.42'. **(OK)**

2.6 MANAGEMENT OF 100 YEAR STORM EVENT

2.6.1 Existing / Pre-Development Site Stormwater Runoff

The pre-developed site's 100-year storm's runoff was generated using the US Army Corps of Engineer's HEC-1 computer model. Pre-Development stormwater runoff calculations are summarized in Appendix A.

Total peak runoff (100 yr storm, pre-development cond.) = 5.76 cfs

2.6.2 Developed Site Stormwater Runoff

A 10 foot wide grassed overflow spillway will be set at elevation 96.50'.

The developed site's 100-year storm's runoff was generated and routed through the stormwater management basin using the US Army Corps of Engineer's HEC-1 computer model. The basin's stage-storage relationship is presented in Article 2.3. For the proposed stage-storage-discharge relationship, the model computed the following values:

Peak basin inflow: 7.50 cfs

Max. water surface in the basin: 96.51'

Peak outflow rate: 0.12 cfs.

A copy of the computer model's output is attached in Appendix A.

2.6.3 Conclusions

The calculations indicate that for the 100-year storm, outflow from the proposed basin will be approximately 0.12 cfs, less than the pre-development peak flow rate of approximately 5.76 cfs. **(OK)**

Maximum water surface in the basin will be approximately 96.51'. **(OK)**

An emergency spillway overflow has been set at 96.50'

Top of embankment will be set at 97.50'.

APPENDIX A

STORMWATER RUNOFF CALCULATIONS

New Expansion Project Parkside Bible Church

491 Eastern Boulevard, Huntington Street
City of Watertown, Jefferson County, New York

ISSUE DATE: 09/17/12 (Rev.-1)

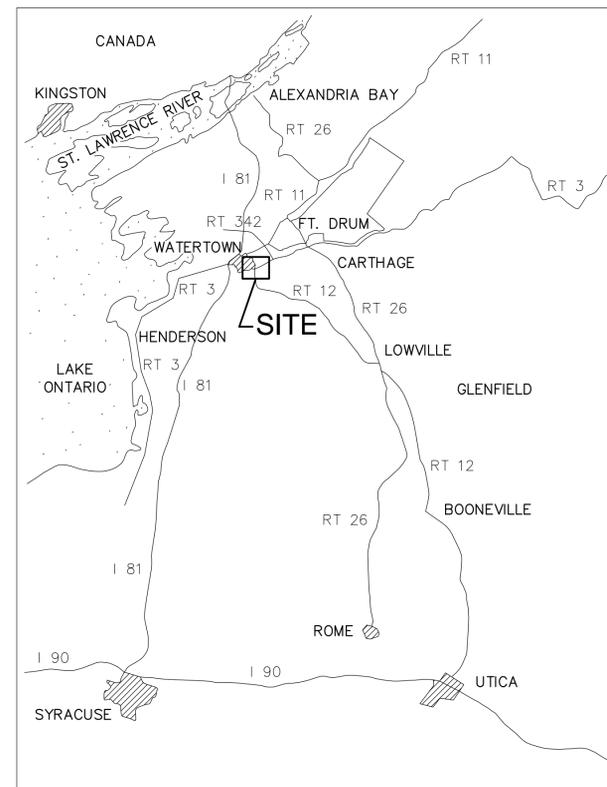
SITE DRAWING LIST

- T100 - TITLE SHEET
- C100 - EXISTING SITE SURVEY
- C101 - SITE LAYOUT PLAN
- C102 - SITE LIGHTING AND UTILITY PLAN
- C103 - SITE GRADING AND DRAINAGE PLAN
- C501 - SITE DETAILS
- C502 - SITE DETAILS
- C503 - SITE DETAILS
- C504 - SITE DETAILS
- C505 - SITE DETAILS



Wilbur D. Thesier, P.E., P.C.
PROFESSIONAL ENGINEER
 P.O. BOX 870
 CARTHAGE, NY 13619
 PHONE: (315) 493-1966

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS ACTING UNDER THE DIRECTION OF LICENSED ARCHITECT, PROFESSIONAL ENGINEER, LANDSCAPE ARCHITECT, OR LAND SURVEYOR TO ALTER ANY ITEM ON THIS DOCUMENT IN ANY WAY. ANY LICENSEE WHO ALTERS THIS DOCUMENT IS REQUIRED BY LAW TO AFFIX HIS OR HER SEAL AND THE NOTATION "ALTERED BY" FOLLOWED BY HIS OR HER SIGNATURE AND SPECIFIC DESCRIPTION OF THE ALTERATIONS.



AREA LOCATION MAP
Not to Scale

Wilbur D. Thesier, P.E. PC
PROFESSIONAL ENGINEER
 PO BOX 870, CARTHAGE, NY 13619
 PHONE: (315) 493-1966

New Expansion Project
Parkside Bible Church
 491 Eastern Boulevard, Huntington Street
 City of Watertown, Jefferson County, New York

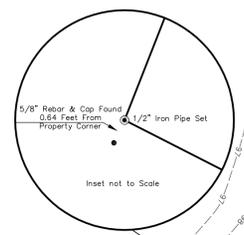
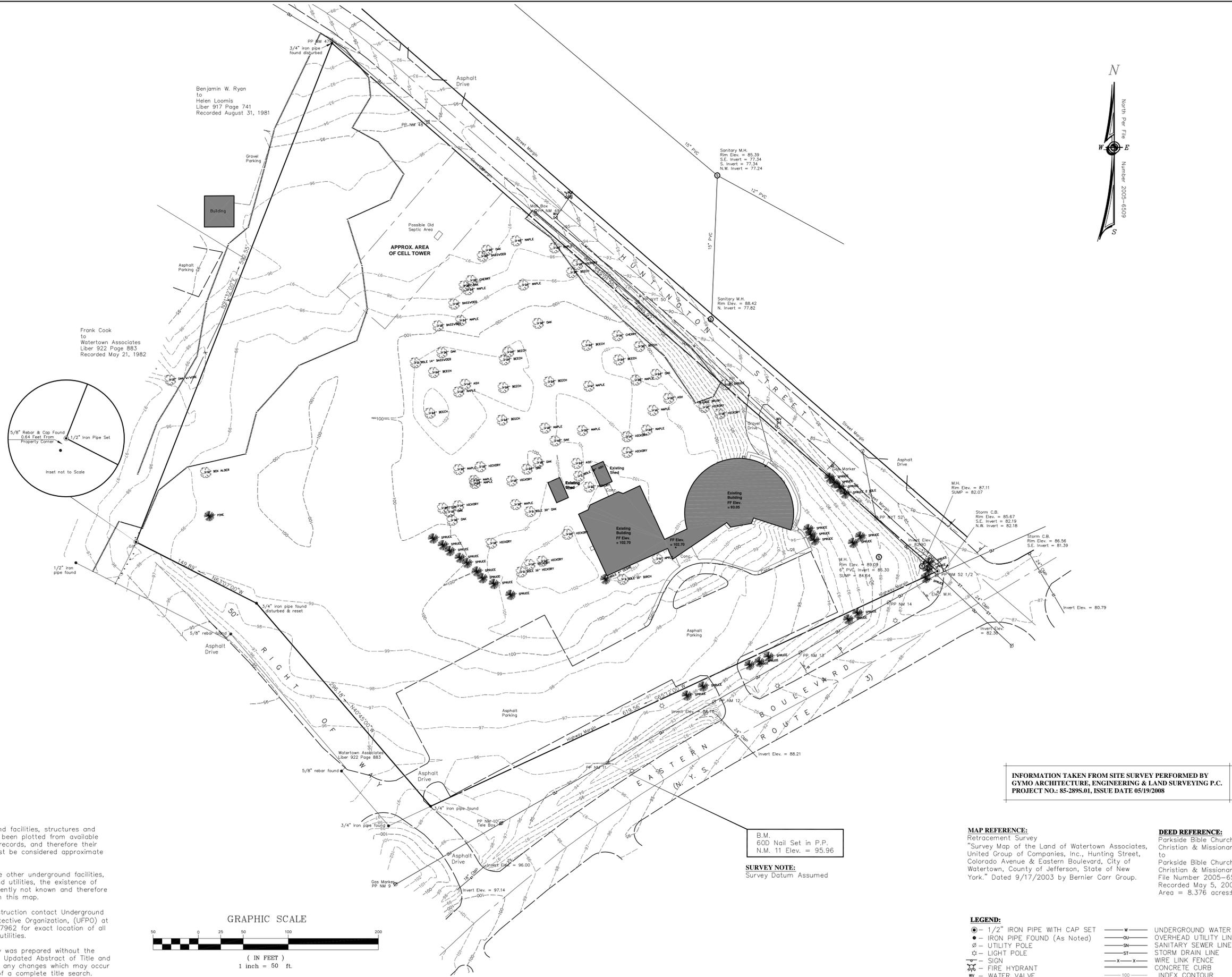
REVISIONS

DATE	DESCRIPTION	BY
09/17/12	REVISION NO.-1	WUL

DATE	08/20/12
DRAWN	W.J.L.
CHECKED	S.O.
SCALE	AS SHOWN

DRAWING NO.
T100

TITLE
PAGE



Frank Cook
to
Watertown Associates
Liber 922 Page 983
Recorded May 21, 1982

Benjamin W. Ryan
to
Helen Loomis
Liber 917 Page 741
Recorded August 31, 1981

Sanitary M.H.
Rim Elev. = 85.39
S.E. Invert = 77.34
S. Invert = 77.34
N.W. Invert = 77.24

Sanitary M.H.
Rim Elev. = 88.42
N. Invert = 77.82

M.H.
Rim Elev. = 87.11
SUMP = 82.07

Storm C.B.
Rim Elev. = 85.67
S.E. Invert = 82.19
N.W. Invert = 82.18

Storm C.B.
Rim Elev. = 86.56
S.E. Invert = 81.39

B.M.
600 Nail Set in P.P.
N.M. 11 Elev. = 95.96

SURVEY NOTE:
Survey Datum Assumed

INFORMATION TAKEN FROM SITE SURVEY PERFORMED BY
GYMO ARCHITECTURE, ENGINEERING & LAND SURVEYING P.C.
PROJECT NO.: 85-2898.01, ISSUE DATE 05/19/2008

MAP REFERENCE:
Retracement Survey
"Survey Map of the Land of Watertown Associates,
United Group of Companies, Inc., Huntington Street,
Colorado Avenue & Eastern Boulevard, City of
Watertown, County of Jefferson, State of New
York." Dated 9/17/2003 by Bernier Carr Group.

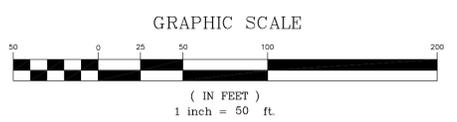
DEED REFERENCE:
Parkside Bible Church of the
Christian & Missionary Alliance
to
Parkside Bible Church of the
Christian & Missionary Alliance
File Number 2005-6509
Recorded May 5, 2005
Area = 8.376 acres±

NOTES:
1. Underground facilities, structures and
utilities have been plotted from available
surveys and records, and therefore their
locations must be considered approximate
only.

There may be other underground facilities,
structures and utilities, the existence of
which is presently not known and therefore
not shown on this map.

Prior to construction contact Underground
Facilities Protective Organization, (UFP0) at
1-800-962-7962 for exact location of all
underground utilities.

2. This survey was prepared without the
benefit of an Updated Abstract of Title and
is subject to any changes which may occur
as a result of a complete title search.



- LEGEND:**
- - 1/2" IRON PIPE WITH CAP SET
 - - IRON PIPE FOUND (As Noted)
 - - UTILITY POLE
 - ☆ - LIGHT POLE
 - ⊕ - SIGN
 - ⊕ - FIRE HYDRANT
 - ⊕ - WATER VALVE
 - W — UNDERGROUND WATER LINE
 - OU — OVERHEAD UTILITY LINES
 - SN — SANITARY SEWER LINE
 - ST — STORM DRAIN LINE
 - X — WIRE LINK FENCE
 - X — CONCRETE CURB
 - 100 — INDEX CONTOUR
 - 101 — CONTOUR

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New Expansion Project
Parkside Bible Church
491 Eastern Boulevard, Huntington Street
City of Watertown, Jefferson County, New York

REVISIONS

DATE	DESCRIPTION	BY

DATE	08/20/12
DRAWN	W.J.L.
CHECKED	S.O.
SCALE	1" = 50'-0"

DRAWING NO.
C100
EXISTING SITE SURVEY



GENERAL SITE NOTES:

1. LOCATION OF UNDERGROUND UTILITIES AND OTHER UNDERGROUND STRUCTURES OBTAINED BY FIELD MEASUREMENTS WHERE VISIBLE AND/OR AVAILABLE UTILITY MAPPING, AND CONSIDERED APPROXIMATE ONLY. OTHER UNDERGROUND UTILITIES AND STRUCTURES MAY EXIST; VERIFY THE LOCATION AND EXTENT OF ALL UTILITY INFORMATION BY CONTACTING THE UNDERGROUND FACILITIES PROTECTIVE ORGANIZATION, INC. AT 1-800-962-7692
2. VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS PRIOR TO CONSTRUCTION. IN CASE OF ANY DISCREPANCY NOTIFY ARCHITECT/ ENGINEER.
3. VERIFY THE CHARACTERISTICS AND EXTENT OF SUBSURFACE SOILS PRIOR TO CONSTRUCTION. IN CASE OF ANY DISCREPANCY, NOTIFY ARCHITECT/ENGINEER.
4. THE PROJECT WORK LIMITS FOR ALL NEW CONSTRUCTION ARE THE SITE BOUNDARY LINES, UNLESS NOTED OTHERWISE.
5. SUPPORT ALL UTILITIES EXPOSED BY EXCAVATION. SUPPORT SYSTEMS TO BE RECOMMENDED BY THE UTILITY AUTHORITY HAVING JURISDICTION.
6. ALL DIMENSIONS ARE TO FACE OF STRUCTURE, OR EDGE OF PAVEMENT, UNLESS NOTED OTHERWISE.
7. PAVED AREAS SHALL BE KEPT CLEAN OF MUD AND DEBRIS AT ALL TIMES.
8. DRAINAGE SHALL BE MAINTAINED AT ALL TIMES.
9. GRADE TO DRAIN AWAY FROM ALL NEW STRUCTURES.
11. BLEND ALL NEW WORK SMOOTHLY WITH EXISTING.
12. REPAIR TO MATCH EXISTING CONDITIONS ALL EXISTING SITE IMPROVEMENTS DISTURBED DURING CONSTRUCTION THAT ARE DESIGNATED TO REMAIN.
13. FINE GRADE AND SEED ALL AREAS DISTURBED BY THIS CONSTRUCTION, WHICH ARE NOT OTHERWISE NOTED.

TAX PARCEL: 5-26-103.007
 OWNER: Parkside Bible Church of C&ME

PROJECT INFORMATION
 CURRENT ZONING: LIGHT INDUSTRY (LI)
 LOT AREA: 8.376 AC± (364,858 S.F.±)
 EXISTING BUILDING AREA: 13,700 S.F.
 NEW BUILDING AREA: 12,500 S.F.
 TOTAL BUILDING AREA: 26,200 S.F.
 TOTAL PAVED AREA: 64,500 S.F.
 TOTAL ON SITE PARKING: 126 SPACES (10x20)

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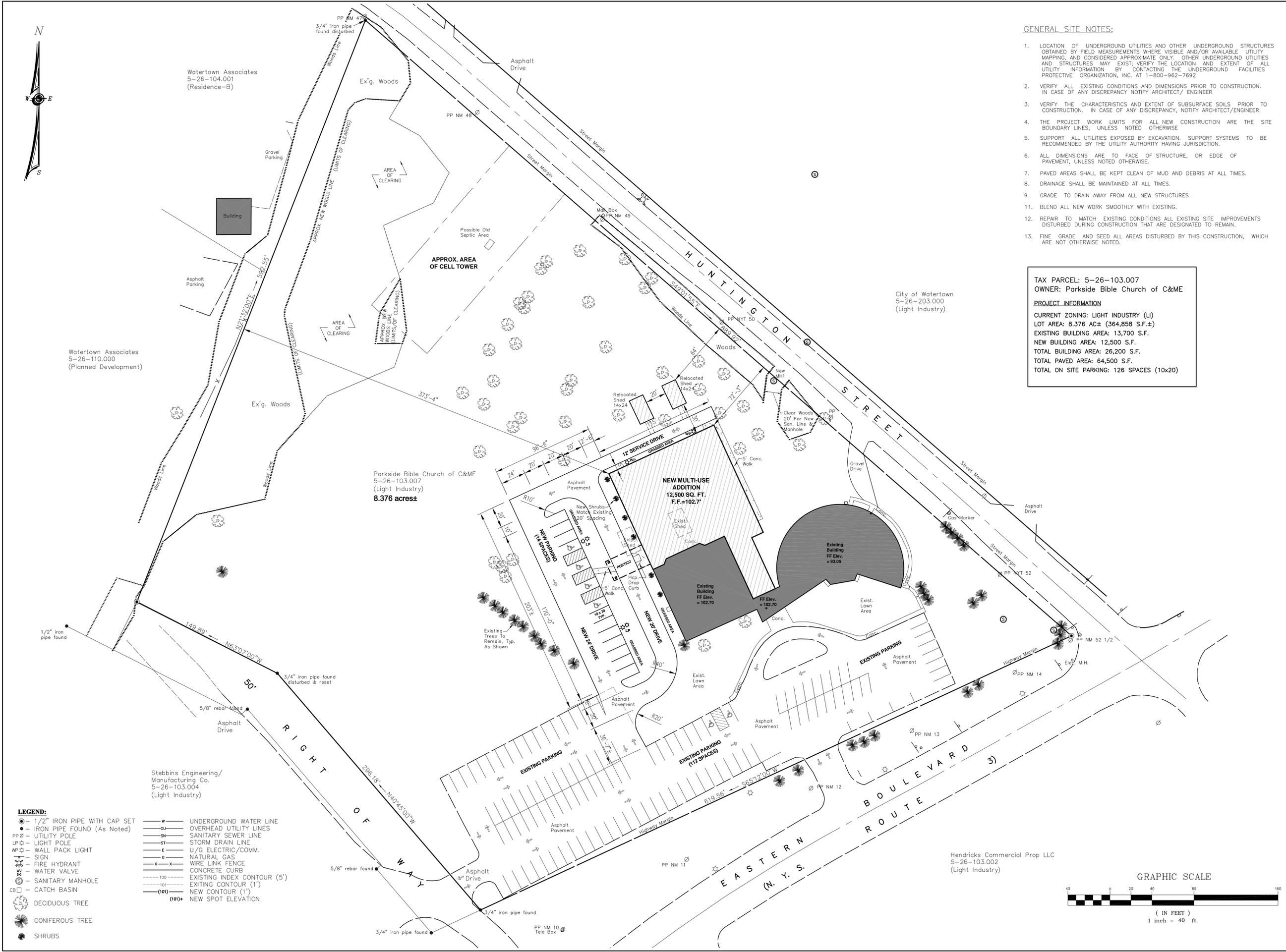
New Expansion Project
Parkside Bible Church
 491 Eastern Boulevard, Huntington Street
 City of Watertown, Jefferson County, New York

REVISIONS

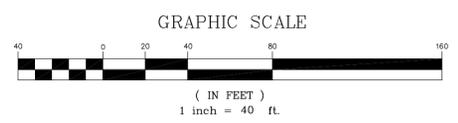
DATE	DESCRIPTION	BY

DATE	08/20/12
DRAWN	W.J.L.
CHECKED	S.O.
SCALE	1" = 40'-0"
DRAWING NO.	C101

SITE LAYOUT PLAN



- LEGEND:**
- 1/2" IRON PIPE WITH CAP SET
 - IRON PIPE FOUND (As Noted)
 - PP ○ UTILITY POLE
 - LP ☆ LIGHT POLE
 - WP ☆ WALL PACK LIGHT
 - ☆ SIGN
 - ☆ FIRE HYDRANT
 - ☆ WATER VALVE
 - SANITARY MANHOLE
 - CB □ CATCH BASIN
 - DECIDUOUS TREE
 - CONIFEROUS TREE
 - SHRUBS
 - w— UNDERGROUND WATER LINE
 - ou— OVERHEAD UTILITY LINES
 - SN— SANITARY SEWER LINE
 - ST— STORM DRAIN LINE
 - e— U/G ELECTRIC/COMM.
 - N— NATURAL GAS
 - x— WIRE LINK FENCE
 - x— CONCRETE CURB
 - 100--- EXISTING INDEX CONTOUR (5')
 - 101--- EXISTING CONTOUR (1')
 - (101)--- NEW CONTOUR (1')
 - (101)+ NEW SPOT ELEVATION





Watertown Associates
5-26-104.001
(Residence-B)

Watertown Associates
5-26-110.000
(Planned Development)

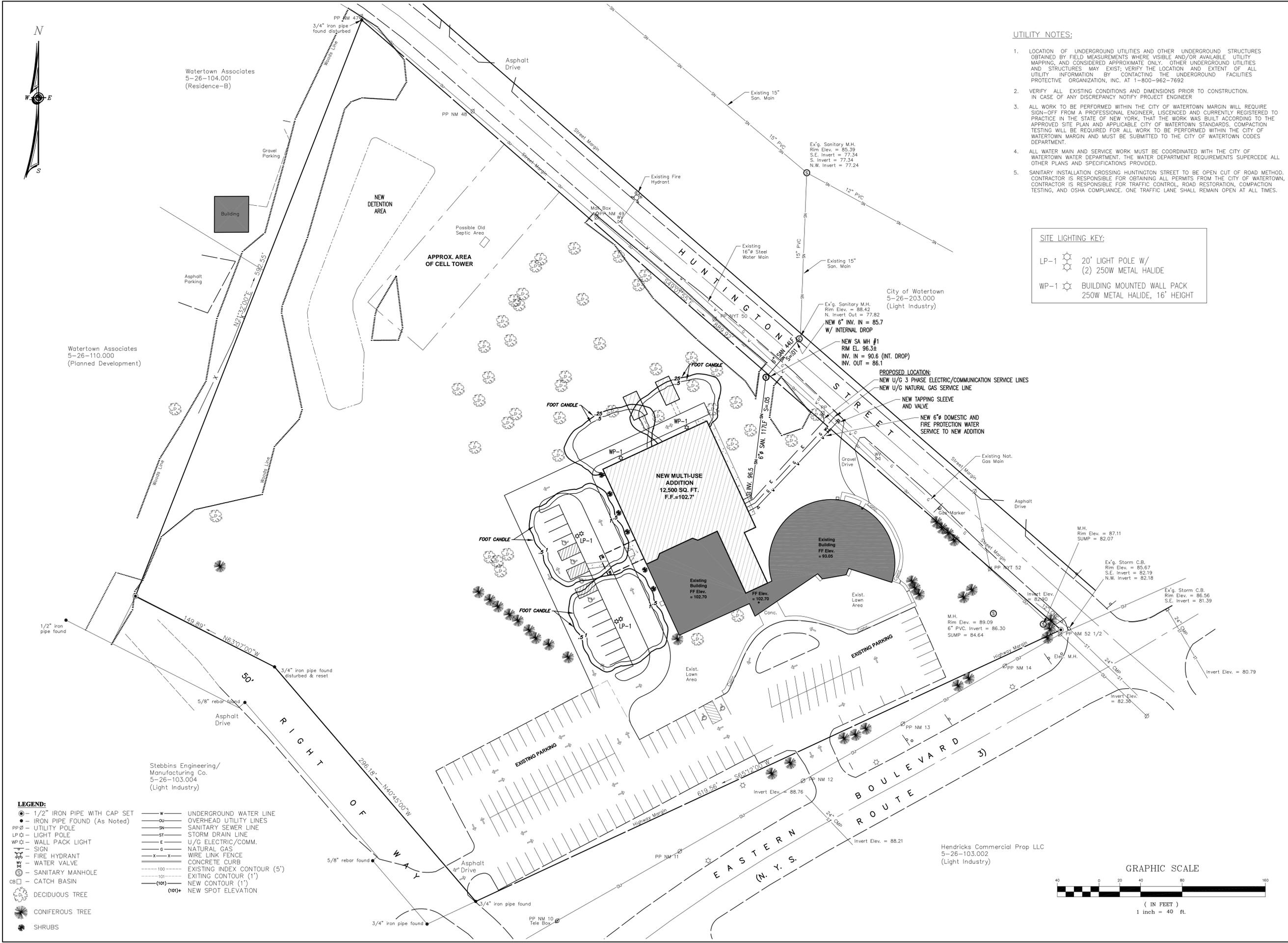
Stebbins Engineering/
Manufacturing Co.
5-26-103.004
(Light Industry)

- LEGEND:**
- 1/2" IRON PIPE WITH CAP SET
 - IRON PIPE FOUND (As Noted)
 - PP ○ UTILITY POLE
 - LP ☆ LIGHT POLE
 - WP ☆ WALL PACK LIGHT
 - ☆ SIGN
 - ☆ FIRE HYDRANT
 - ☆ WATER VALVE
 - SANITARY MANHOLE
 - CB □ CATCH BASIN
 - DECIDUOUS TREE
 - CONIFEROUS TREE
 - SHRUBS
 - w UNDERGROUND WATER LINE
 - ou OVERHEAD UTILITY LINES
 - sn SANITARY SEWER LINE
 - st STORM DRAIN LINE
 - e U/G ELECTRIC/COMM.
 - g NATURAL GAS
 - x-x WIRE LINK FENCE
 - CONCRETE CURB
 - EXISTING INDEX CONTOUR (5')
 - EXISTING CONTOUR (1')
 - NEW CONTOUR (1')
 - (101)+ NEW SPOT ELEVATION

UTILITY NOTES:

- LOCATION OF UNDERGROUND UTILITIES AND OTHER UNDERGROUND STRUCTURES OBTAINED BY FIELD MEASUREMENTS WHERE VISIBLE AND/OR AVAILABLE UTILITY MAPPING, AND CONSIDERED APPROXIMATE ONLY. OTHER UNDERGROUND UTILITIES AND STRUCTURES MAY EXIST; VERIFY THE LOCATION AND EXTENT OF ALL UTILITY INFORMATION BY CONTACTING THE UNDERGROUND FACILITIES PROTECTIVE ORGANIZATION, INC. AT 1-800-962-7692
- VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS PRIOR TO CONSTRUCTION. IN CASE OF ANY DISCREPANCY NOTIFY PROJECT ENGINEER
- ALL WORK TO BE PERFORMED WITHIN THE CITY OF WATERTOWN MARGIN WILL REQUIRE SIGN-OFF FROM A PROFESSIONAL ENGINEER, LICENSED AND CURRENTLY REGISTERED TO PRACTICE IN THE STATE OF NEW YORK. THAT THE WORK WAS BUILT ACCORDING TO THE APPROVED SITE PLAN AND APPLICABLE CITY OF WATERTOWN STANDARDS. COMPACTION TESTING WILL BE REQUIRED FOR ALL WORK TO BE PERFORMED WITHIN THE CITY OF WATERTOWN MARGIN AND MUST BE SUBMITTED TO THE CITY OF WATERTOWN CODES DEPARTMENT.
- ALL WATER MAIN AND SERVICE WORK MUST BE COORDINATED WITH THE CITY OF WATERTOWN WATER DEPARTMENT. THE WATER DEPARTMENT REQUIREMENTS SUPERCEDE ALL OTHER PLANS AND SPECIFICATIONS PROVIDED.
- SANITARY INSTALLATION CROSSING HUNTINGTON STREET TO BE OPEN CUT OF ROAD METHOD. CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS FROM THE CITY OF WATERTOWN. CONTRACTOR IS RESPONSIBLE FOR TRAFFIC CONTROL, ROAD RESTORATION, COMPACTION TESTING, AND OSHA COMPLIANCE. ONE TRAFFIC LANE SHALL REMAIN OPEN AT ALL TIMES.

- SITE LIGHTING KEY:**
- LP-1 ☆ 20' LIGHT POLE W/
(2) 250W METAL HALIDE
 - WP-1 ☆ BUILDING MOUNTED WALL PACK
250W METAL HALIDE, 16' HEIGHT



Wilbur D. Thesier, P.E. PC
PROFESSIONAL ENGINEER
PO BOX 870, CARTHAGE, NY 13619
PHONE: (315) 493-1966

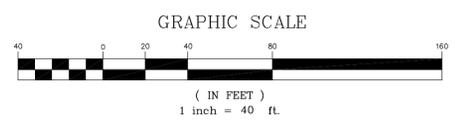
New Expansion Project
Parkside Bible Church
491 Eastern Boulevard, Huntington Street
City of Watertown, Jefferson County, New York

REVISIONS

DATE	DESCRIPTION	BY
09/17/12	REVISED NEW SANITARY INVERTS	WUL

DATE	08/20/12
DRAWN	W.J.L.
CHECKED	S.O.
SCALE	1" = 40'-0"

DRAWING NO.
C102
SITE LIGHTING AND UTILITY PLAN



Hendricks Commercial Prop LLC
5-26-103.002
(Light Industry)



Watertown Associates
5-26-104.001
(Residence-B)

Watertown Associates
5-26-110.000
(Planned Development)

Stebbins Engineering/
Manufacturing Co.
5-26-103.004
(Light Industry)

City of Watertown
5-26-203.000
(Light Industry)

Hendricks Commercial Prop LLC
5-26-103.002
(Light Industry)

EARTHWORK NOTES:

1. DURING CONSTRUCTION AND UNTIL DISTURBED AREAS ARE STABILIZED, MAINTAIN THE SITE AGAINST THE FORCES OF WATER AND WIND EROSION.
2. RESTRICT GRADING OPERATIONS TO AREAS SHOWN ON THE CONSTRUCTION DRAWINGS. GRADING OUTSIDE THE PROJECT WORK LIMITS WILL NOT BE PERMITTED.
3. BACKFILL ALL EXCAVATIONS WITH COMPACTED GRANULAR FILL IN 6" LIFTS TO 95% DENSITY.
4. TOPSOIL REQUIRED FOR ESTABLISHMENT OF VEGETATION SHALL BE STOCKPILED IN AMOUNT NECESSARY TO COMPLETE FINISH GRADING OF ALL EXPOSED AREAS. ANY EXCESS TOPSOIL SHALL BE SPREAD ON-SITE OR REMOVED FROM SITE UPON COMPLETION OF EARTHWORK OPERATIONS.
5. AREAS TO BE FILLED SHALL BE CLEARED, GRUBBED AND STRIPPED OF TOPSOIL TO REMOVE VEGETATION, ROOTS, OR OTHER EXTRANEIOUS MATERIAL PRIOR TO PLACING FILL.
6. REMOVE FROM SITE AND DISPOSE OF ALL EXCESS EXCAVATED MATERIAL NOT USED ELSEWHERE.
7. FROZEN MATERIAL OR SOFT, ORGANIC OR HIGHLY COMPRESSIBLE MATERIAL SHALL NOT BE INCORPORATED INTO FILL SLOPES OR STRUCTURAL FILLS.
8. SEEPS OR SPRINGS ENCOUNTERED DURING CONSTRUCTION SHALL BE HANDLED IN ACCORDANCE WITH APPROVED DEWATERING METHODS.

Wilbur D. Thesier, P.E. PC
PROFESSIONAL ENGINEER
PO BOX 870, CARTHAGE, NY 13619
PHONE: (315) 493-1966

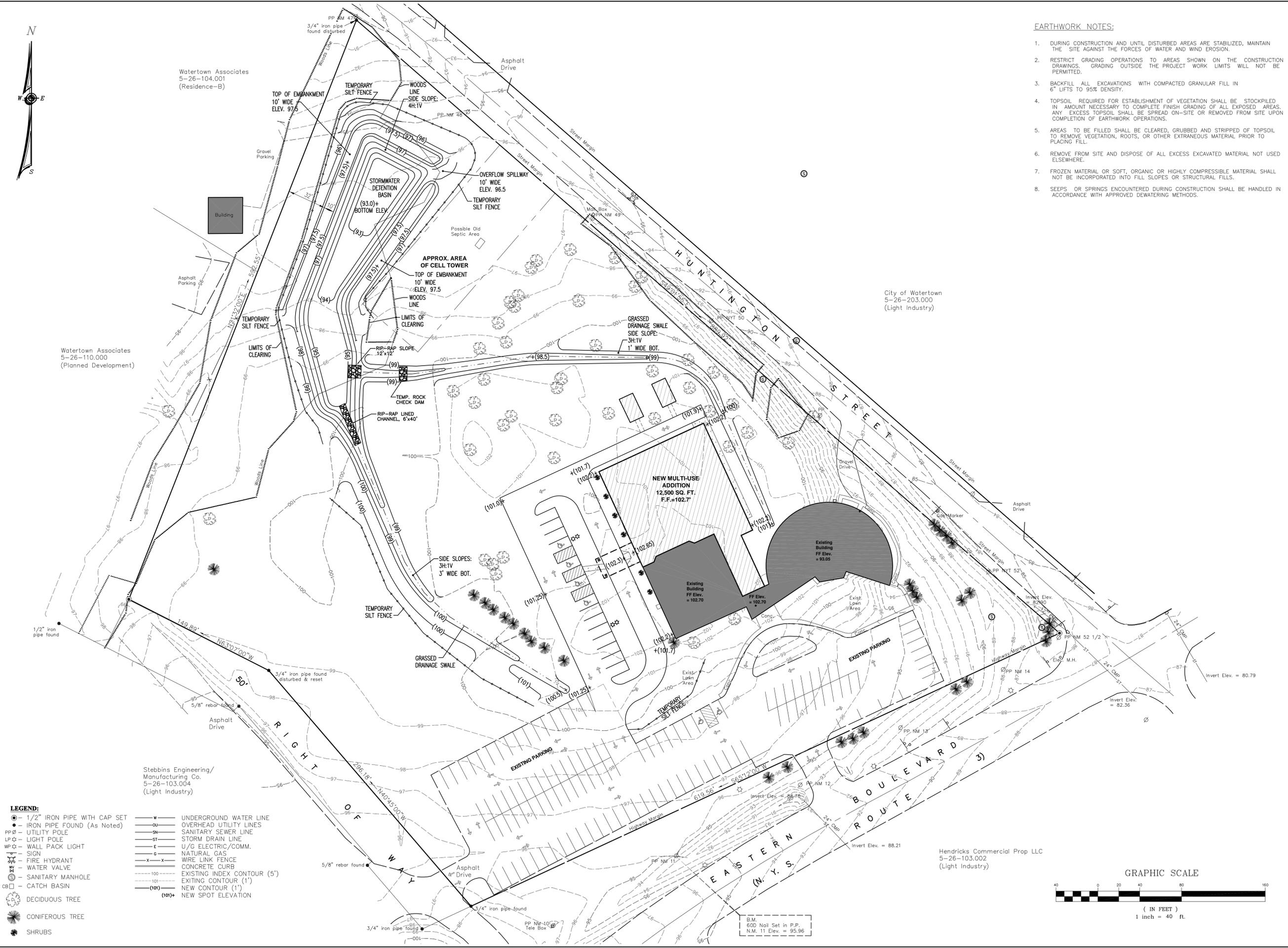
New Expansion Project
Parkside Bible Church
491 Eastern Boulevard, Huntington Street
City of Watertown, Jefferson County, New York

REVISIONS		
DATE	DESCRIPTION	BY
09/17/12	REVISED GRADING AND DRAINAGE	WUL

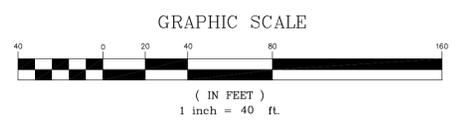
DATE	08/20/12
DRAWN	W.J.L.
CHECKED	S.O.
SCALE	1" = 40'-0"
DRAWING NO.	

C103

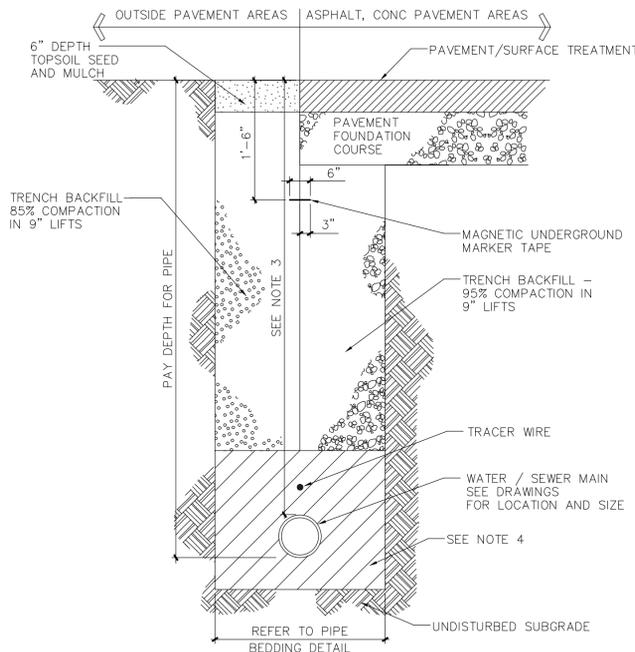
SITE GRADING AND DRAINAGE PLAN



- LEGEND:**
- 1/2" IRON PIPE WITH CAP SET
 - IRON PIPE FOUND (As Noted)
 - PP ○ UTILITY POLE
 - LP ☆ LIGHT POLE
 - WP ☆ WALL PACK LIGHT
 - ☆ FIRE HYDRANT
 - ☆ WATER VALVE
 - SANITARY MANHOLE
 - CB □ CATCH BASIN
 - DECIDUOUS TREE
 - CONIFEROUS TREE
 - SHRUBS
 - w UNDERGROUND WATER LINE
 - ou OVERHEAD UTILITY LINES
 - sn SANITARY SEWER LINE
 - st STORM DRAIN LINE
 - e U/G ELECTRIC/COMM.
 - g NATURAL GAS
 - x—x— WIRE LINK FENCE
 - x—x— CONCRETE CURB
 - 100--- EXISTING INDEX CONTOUR (5')
 - 101--- EXISTING CONTOUR (1')
 - (101)--- NEW CONTOUR (1')
 - (101)+ NEW SPOT ELEVATION

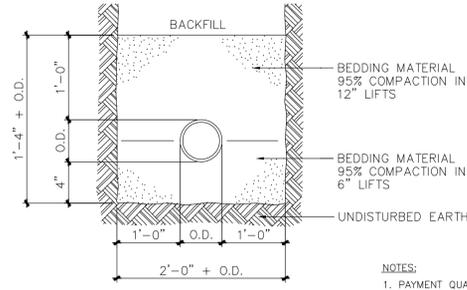


B.M.
60D Nail Set in P.P.
N.M. 11 Elev. = 95.96

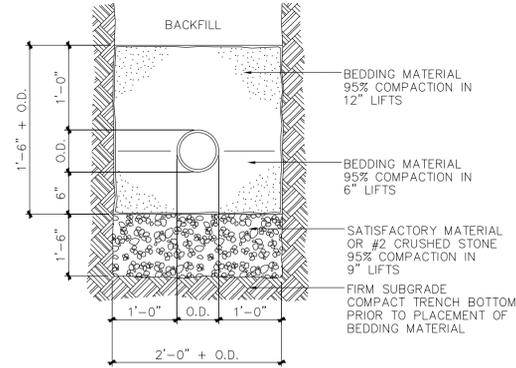


- NOTES:**
- REFER TO APPROPRIATE DETAILS FOR PAVEMENT DEPTHS.
 - TRENCH DEPTHS, WIDTHS, SHEETING, SHORING, BRACING AND CUTBACK SLOPES TO BE DETERMINED BY THE CONTRACTOR AND SHALL COMPLY WITH O.S.H.A., NEW YORK STATE DEPARTMENT OF LABOR, NEW YORK STATE INDUSTRIAL CODE AND ALL OTHER APPLICABLE SAFETY STANDARDS.
 - DEPTH 5'-6" MIN BURIAL DEPTH FOR WATER LINES.
 - SEE DETAILS FOR BEDDING.

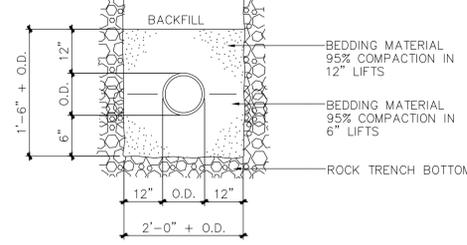
1 TYPICAL PIPE TRENCH DETAIL
SCALE: NONE



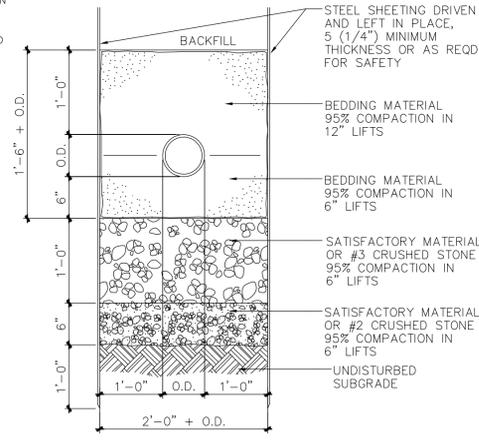
- NOTES:**
- PAYMENT QUANTITIES FOR SELECT FILL AND ASPHALT REPLACEMENT SHALL BE CALCULATED BASED ON MEASUREMENTS USING NEAT LINE LIMITS SHOWN HEREIN. CONTRACTOR SHALL BASE HIS UNIT PRICE ON PAYMENT LIMITS. WORK OUTSIDE THESE LIMITS SHALL NOT BE CONSIDERED FOR PAYMENT.
 - CONTRACTOR SHALL BE RESPONSIBLE FOR RESTORING CULTIVATED AND UNCULTIVATED AREAS DISTURBED DURING CONSTRUCTION AS DESCRIBED IN THE CONTRACT DRAWINGS.



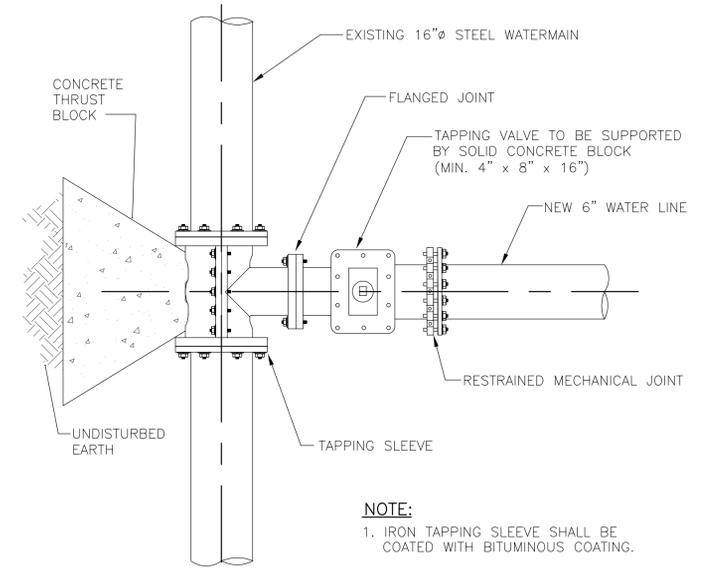
SPECIAL BEDDING - MODERATELY STABLE SOIL



SPECIAL BEDDING - ROCK TRENCH

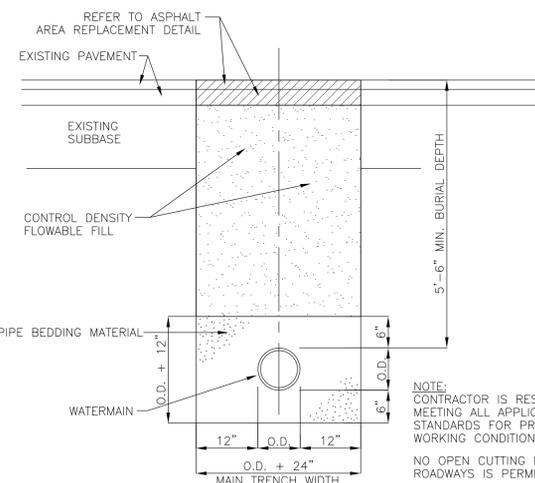


SPECIAL BEDDING - UNSTABLE SOIL



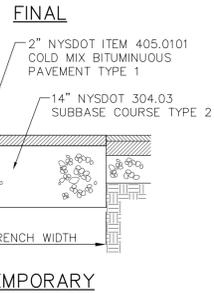
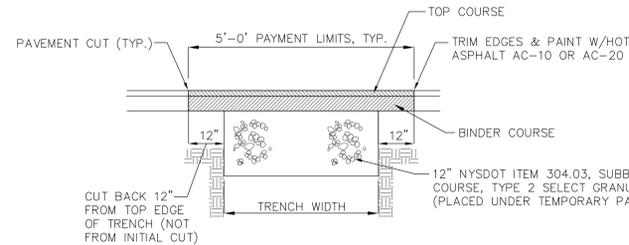
- NOTE:**
- IRON TAPPING SLEEVE SHALL BE COATED WITH BITUMINOUS COATING.

3 TYP. TAPPING SLEEVE AND VALVE
SCALE: NONE
(DI, PVC, CI, AC PIPE)



- NOTE:** CONTRACTOR IS RESPONSIBLE FOR MEETING ALL APPLICABLE SAFETY STANDARDS FOR PROVIDING SAFE WORKING CONDITIONS
- NO OPEN CUTTING IN NYS DOT ROADWAYS IS PERMITTED

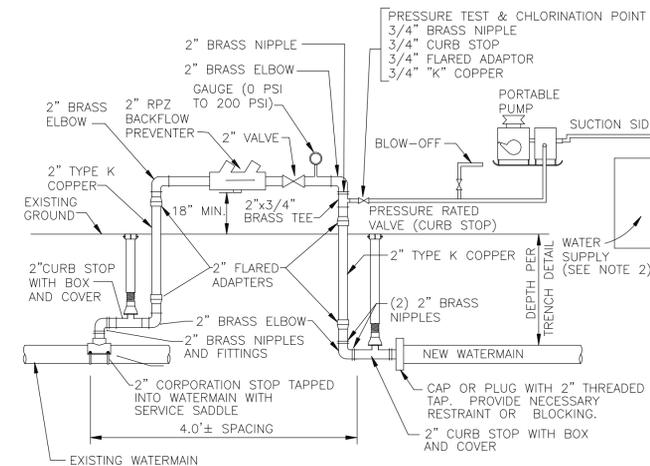
4 TYPICAL CITY ROAD CROSSING DETAIL
SCALE: NONE



- NOTE:** TEMPORARY PAVEMENT TO BE MAINTAINED FOR 30 DAYS PRIOR TO PLACEMENT OF FINAL PAVEMENT.

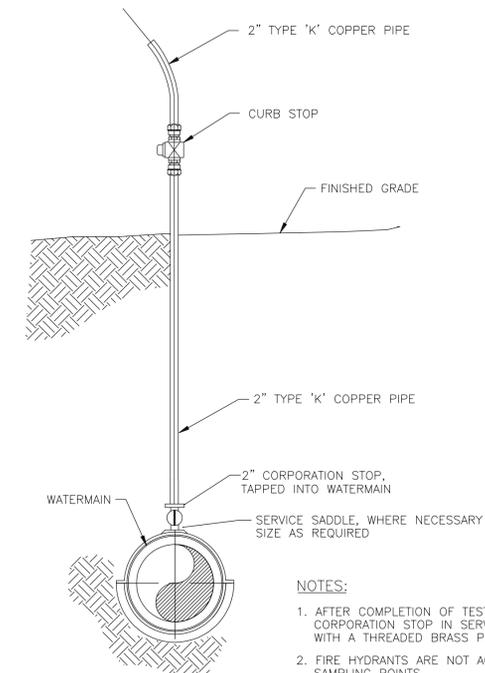
AREA FOR REPLACEMENT	TOP COURSE	BINDER COURSE
MAJOR COMMERCIAL	1 1/2"	4"
MINOR COMMERCIAL	1 1/2"	3"
RESIDENTIAL DRIVEWAY	1 1/2"	2 1/2"
VILLAGE, TOWN, AND COUNTY ROAD	2"	4"

5 ASPHALT AREA REPLACEMENT DETAIL
SCALE: NONE
(EXCEPT FOR NYS DOT RIGHT-OF-WAY)



- NOTES:**
- BACTERIOLOGICAL WATER SAMPLES SHALL BE TAKEN AT WATERMAIN BEGINNING AND END STATIONS AND AT MAXIMUM 1000 FT. SPACING.
 - FOR PRESSURE TESTING PROVIDE CLEAN WATER SUPPLY. FOR DISINFECTION PROVIDE CHLORINATED WATER SUPPLY.

6 PRESSURE TESTING AND DISINFECTION APPARATUS
SCALE: NONE
(TAP TO EXISTING WATERMAIN)



- NOTES:**
- AFTER COMPLETION OF TEST, REPLACE CORPORATION STOP IN SERVICE SADDLE WITH A THREADED BRASS PLUG.
 - FIRE HYDRANTS ARE NOT ACCEPTABLE SAMPLING POINTS.

7 TEMPORARY BLOW-OFF/SAMPLING POINT
SCALE: NONE

Wilbur D. Thesier, P.E. PC
PROFESSIONAL ENGINEER
PO BOX 870, CARTHAGE, NY 13619
PHONE: (315) 493-1966

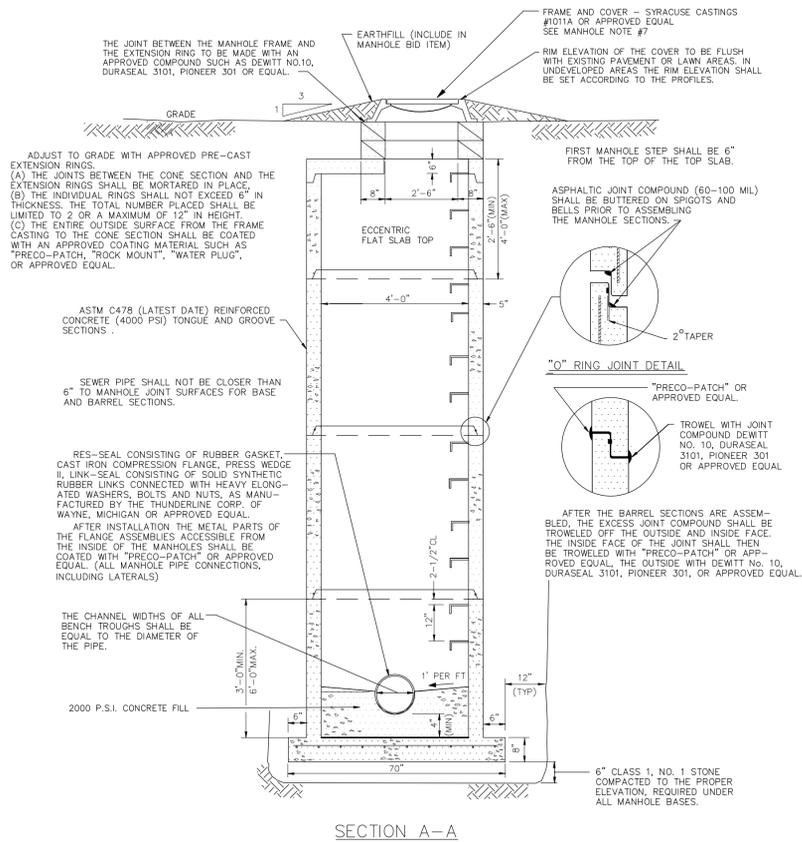
New Expansion Project
Parkside Bible Church
491 Eastern Boulevard, Huntington Street
City of Watertown, Jefferson County, New York

REVISIONS		
DATE	DESCRIPTION	BY

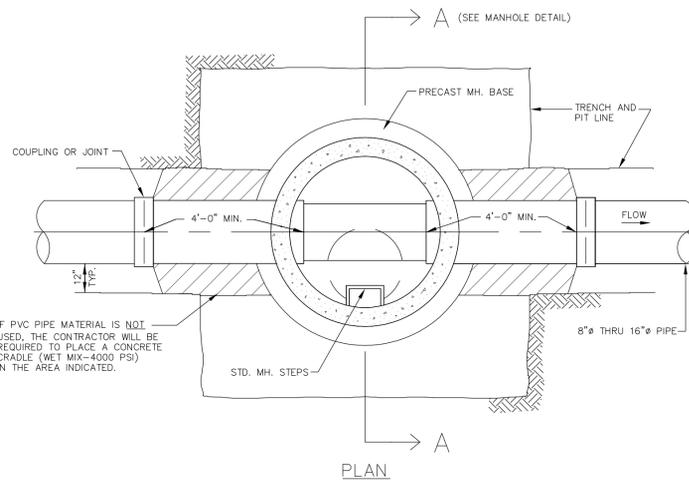
DATE	08/20/12
DRAWN	W.J.L.
CHECKED	S.O.
SCALE	AS SHOWN

DRAWING NO.
C501

SITE DETAILS



SECTION A-A



PLAN

1 MONOLITHIC PRECAST BASE MANHOLES
C503 SCALE: NONE (4'-0"Ø FOR SEWERS 16"Ø AND LESS)

SANITARY SEWER GENERAL NOTES

GENERAL:

- THE CONTRACTOR SHALL NOT COMMENCE WORK UNTIL HE HAS SUBMITTED MATERIALS CERTIFICATES TO THE ENGINEER AND THESE HAVE BEEN RETURNED TO HIM STAMPED APPROVED. IN ADDITION, 3 COPIES OF THE MATERIAL SUBMITTALS SHALL BE PROVIDED TO THE CITY FOR REVIEW & APPROVAL PRIOR TO PROCEEDING MATERIAL.
- CONTRACTOR SHALL NOTIFY THE ENGINEER AND THE CITY INSPECTOR AT LEAST 24 HOURS PRIOR TO STARTING ANY WORK.
- CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS RELATING TO THE WORK.

GRAVITY PIPE:

- PIPE SHALL BE PVC TYPE PSM, MEETING ASTM D-3034, LATEST EDITION
- PIPE STANDARD SHALL BE SDR-35. MINIMUM AND JOINTS SHALL BE RING-TITE, MEETING ASTM D-1869, LATEST EDITION
- BELL ENDS TO BE LAID UPHILL.
- NO CELLAR DRAIN, SUMP PUMPS OR ROOF DRAINS SHALL BE CONNECTED TO THE SANITARY SEWER.
- SANITARY LINE SHALL NOT BE COVERED UNTIL INSPECTED BY THE ENGINEER OR THE CITY INSPECTOR.
- WHERE GROUND WATER IS PRESENT, CLAY DAMS SHALL BE CONSTRUCTED AT 100 FT. INTERVALS. THE DAMS SHALL BE A MINIMUM OF 18 INCHES THICK AND EXTEND THE FULL HEIGHT AND WIDTH OF THE TRENCH.

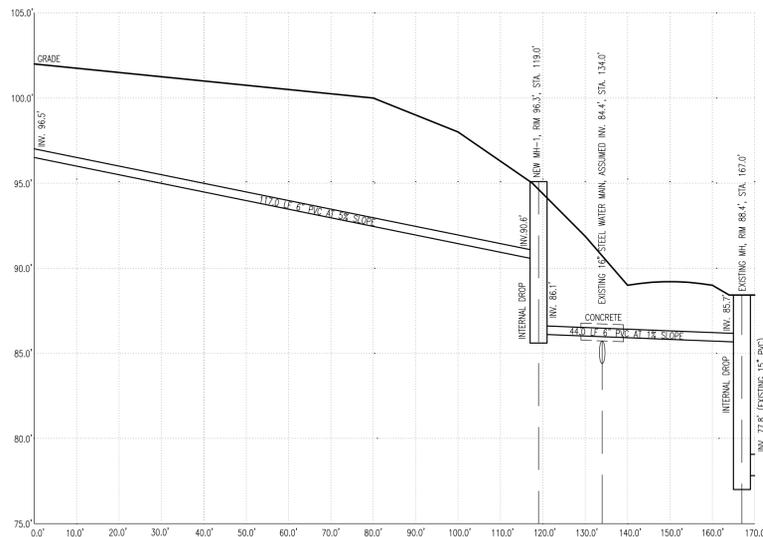
MANHOLES:

- MANHOLES SHALL BE PRECAST REINFORCED CONCRETE STRUCTURES MEETING ASTM C 478
- STEPS SHALL BE IN ALIGNMENT VERTICALLY AND CENTERED OVER A BENCH AREA.
- STEPS SHALL PROJECT A MINIMUM OF 5 INCHES BEYOND THE INTERIOR OF THE MANHOLE WALL.
- ALL INTERIOR SANITARY MANHOLES SHALL BE PARGED AND ALL EXTERIOR JOINTS SEALED WITH AN APPROVED ASPHALT COATING. HEAVY BLACK BITUMASTIC (14 MILS) ENTIRE EXTERIOR INCLUDING JOINTS, BRICK, ADJUSTMENTS AND TOP SLABS.
- THE BOTTOM OF THE MANHOLE SHALL BE SHAPED IN ACCORDANCE WITH LOCAL CODES AND SPECIFICATIONS. FORM INVERTS WITH CONCRETE FROM TOP OF PIPE TO SPRINGLINE. FLOW CHANNEL TO BE SMOOTH GROUT.
- THE MANHOLE TO SANITARY CONNECTION WILL BE MADE AS SHOWN IN DETAIL ON THIS SHEET. THE WATER TIGHT CONNECTION WILL BE ACCOMPLISHED THROUGH THE USE OF A STANDARD ELASTIC BOOT CAST INTO STRUCTURE WALLS AS PER A.S.T.M.C. - 443.
- MANHOLE FRAME AND COVERS SHALL BE IMPRINTED WITH THE CITY NAME AND THE WORDS "SANITARY SEWER". IN 2 INCH LETTERS. PROVIDE 2 WATER TIGHT PICK HOLES A48 - B3 CLASS 30B FOR ASH TO LOAD HS20 - S16.
- THE JOINTS BETWEEN THE MANHOLE SECTIONS WILL BE AN O-RING ASTM C443 AND MEETING NYSDOT ITEM 706.02 OR BUTYL RESIN ASHTO SPEC.M198B.

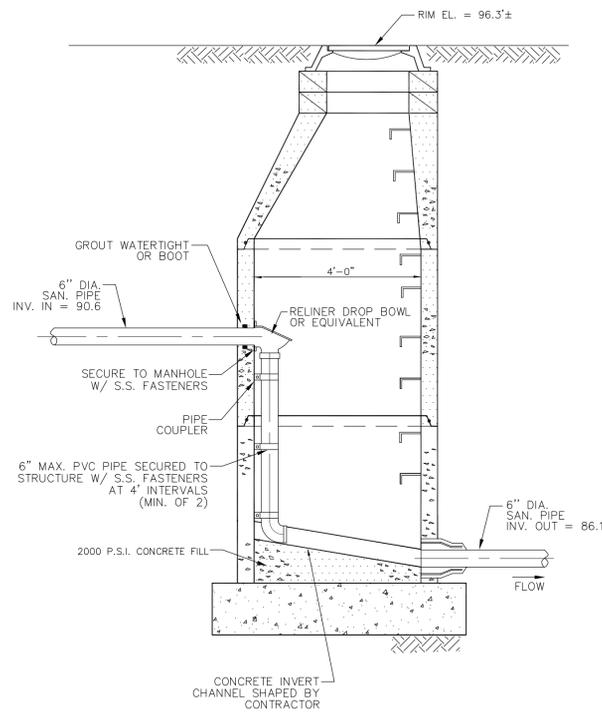
TESTING:

- TESTING SHALL MEET THE CURRENT REQUIREMENTS OF THE CITY OR AS SET FORTH BELOW. ALL TESTING MUST BE WITNESSED BY THE ENGINEER AND CITY OFFICIAL.
- PIGGING (DEFLECTION) TESTING
ALL LINES SHALL HAVE A PIGGING DEVICE PULLED THROUGH THEM CAPABLE OF TESTING THE ROUNDNESS OF THE INSTALLED LINE. THE PIG SHALL BE A RIGID CIRCULAR OR SPHERICAL DEVICE WITH AN OUTSIDE DIAMETER EQUAL TO 95% OF THE INSIDE DIAMETER OF THE PIPE TO BE TESTED.
- LAMPING (ALIGNMENT) TESTING
ALL LINES SHALL BE VISUALLY LAMPED FOR ALIGNMENT, A FULL MOON MUST BE VISIBLE.
- AIR PRESSURE (LEAKAGE) PIPE TESTING
ALL SANITARY SEWER LINES SHALL BE PRESSURE TESTED IN ACCORDANCE WITH ASTM F-1417.
- ALL SANITARY SEWER MANHOLES SHALL BE TESTED FOR LEAKAGE USING ONE OF THE FOLLOWING METHODS:
A) BY PLUGGING THE OUTLET LINE AND FILLING THE MANHOLE WITH WATER TO A POINT AT LEAST ONE FOOT ABOVE THE UPPERMOST JOINT (EXCLUDING THE GRADE ADJUSTMENT COURSES). AFTER 24 HOURS, THE DROP IN THE WATER LEVEL SHALL BE MEASURED AND THE LOSS CALCULATED. THE ALLOWABLE LOSS SHALL BE EQUAL TO 100 GALLONS/INCH/MILE/DAY.B) VACUUM TESTED IN ACCORDANCE WITH APPLICABLE ASTM STANDARDS.

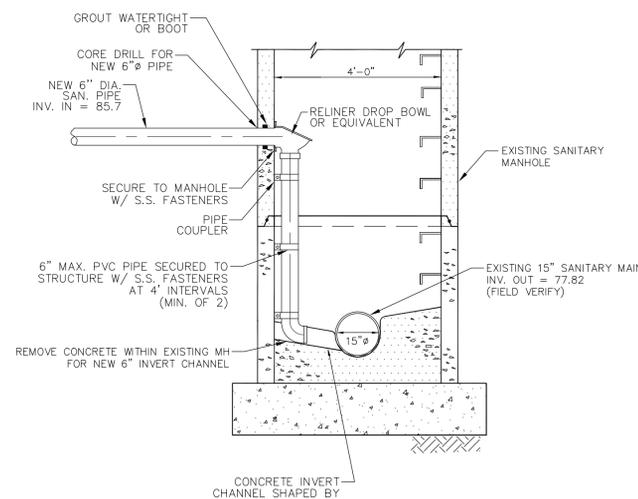
NOTE:
ALL CONSTRUCTION, INSPECTION AND MONITORING SHALL COMPLY WITH ALL OSHA, STATE AND LOCAL ORDINANCES, LAWS AND PROCEDURES. THIS SHALL INCLUDE ALL CONSTRUCTION METHODS, MATERIALS AND SAFETY PROCEDURES. ADDITIONAL TRENCH SAFETY, INSPECTION AND MONITORING SHALL BE DESIGNED AND FURNISHED BY THE CONTRACTOR AS REQUIRED BY OSHA, STATE AND LOCAL ORDINANCES, LAWS AND PROCEDURES.



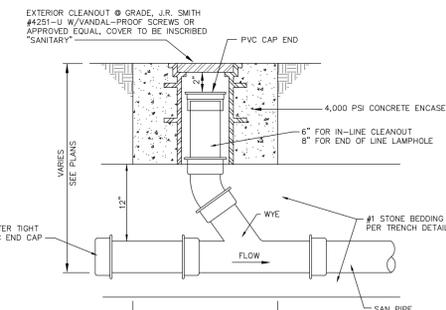
2 SANITARY PROFILE
C504 SCALE: HORIZ. 1"=20', VERT. 1"=5'



3 SANITARY MANHOLE #1 DETAIL (INTERNAL DROP)
C503 SCALE: NONE



4 CONN. TO EXISTING SANITARY M.H. (INTERNAL DROP)
C503 SCALE: NONE



5 CLEANOUT TO GRADE
C504 SCALE: NONE

Wilbur D. Thesier, P.E. PC
PROFESSIONAL ENGINEER
PO BOX 870, CARTHAGE, NY 13619
PHONE: (315) 493-1966

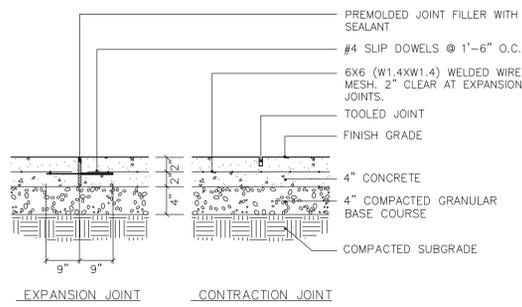
New Expansion Project
Parkside Bible Church
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REVISIONS		
DATE	DESCRIPTION	BY
09/17/12	REVISED NEW SANITARY	WJL

DATE	08/20/12
DRAWN	W.J.L.
CHECKED	S.O.
SCALE	AS SHOWN

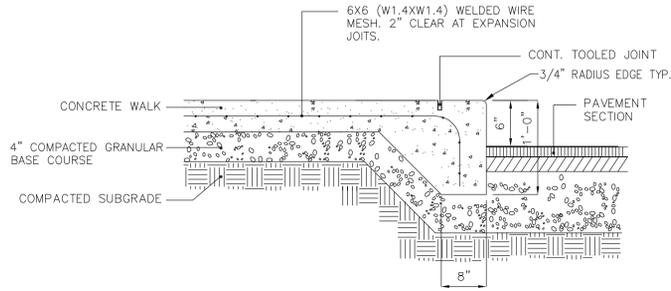
DRAWING NO. **C503**

SITE DETAILS

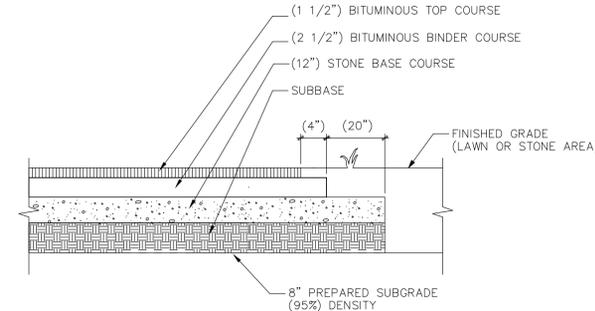


NOTE-
1. CONTROL JOINTS TO BE SPACED 5' O.C. MAX., EXPANSION JOINTS TO BE SPACED 30' O.C. MAX.

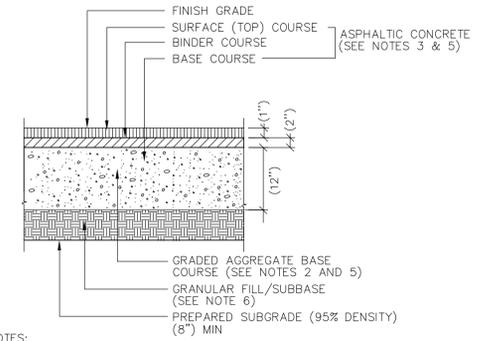
1 CONCRETE WALK DETAIL
C504 SCALE: NONE



2 CONCRETE WALK/INTEGRAL CURB DETAIL
C504 SCALE: NONE



3 PAVEMENT TO LAWN TRANSITION
C504 SCALE: NONE



NOTES:
1. CONSTRUCT ASPHALT SURFACE OR BITUMINOUS SURFACE TREATMENT ONLY WHEN ATMOSPHERIC TEMPERATURE IS ABOVE 50 DEGREES F. AND RISING, AND WHEN BASE IS DRY. BINDER COURSE MAY BE PLACED WHEN AIR TEMPERATURE IS ABOVE 45 DEGREES F. AND RISING.
2. GRANULAR BASE COURSE SHALL CONFORM TO NYS DOT STANDARD SPECIFICATION SECTION 304 OPTION C - TYPE 2 FOR CRUSHED STONE, NYS DOT ITEM NO. 304 12M, AS SPECIFIED BELOW:

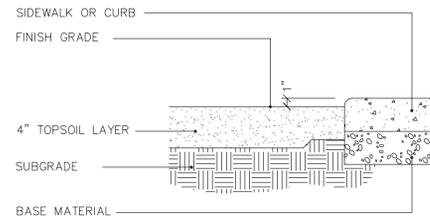
STANDARD SIEVE SIZE	PERCENT PASSING BY WEIGHT
(2")	100%
(1/4")	25-60%
425um	5-40%
75um	0-5%

3. ASPHALTIC CONCRETE SHALL CONFORM TO NYS DOT STANDARD SPECIFICATIONS, SECTION 403; SURFACE COURSE TYPE 7F, NYS DOT ITEM NO. 403.198202M; BINDER COURSE TYPE 3, NYS DOT ITEM NO. 403.138902M.
4. APPLY TACK COAT OF ASPHALTIC CEMENT TO ALL VERTICAL JOINTS BETWEEN OLD AND NEW WORK. USE NYS DOT ITEMS NO. 407.01M MEDIUM SETTING.
5. TEST FINISHED SURFACE OF EACH COURSE FOR SMOOTHNESS, USING (10') STRAIGHTEDGE APPLIED PARALLEL WITH, AND AT RIGHT ANGLES TO CENTERLINE OF PAVED AREAS. SURFACES WILL NOT BE ACCEPTABLE IF EXCEEDING THE FOLLOWING TOLERANCES FOR SMOOTHNESS:

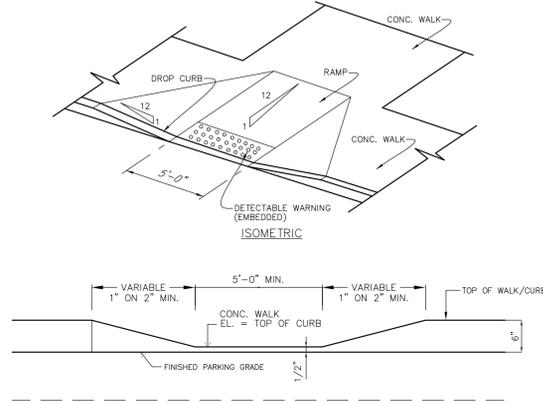
BASE COURSE SURFACE: (1/2")
BINDER COURSE SURFACE: (1/4")
SURFACE COURSE SURFACE: (1/4")

6. GRANULAR FILL/SUBBASE SHALL BE (12") MIN.

4 ASPHALT PAVEMENT DETAIL
C504 SCALE: NONE

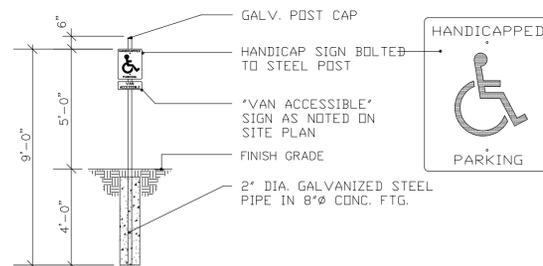
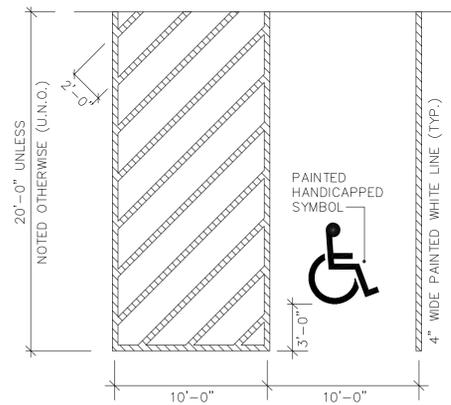
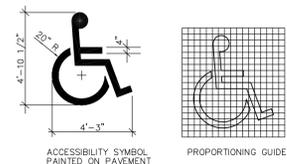


5 GRASSED AREAS DETAIL
C504 SCALE: NONE



7 SHRUB PLANTING DETAIL
C504 SCALE: NONE

6 ACCESSIBLE RAMP DETAIL
C504 SCALE: NONE



8 ACCESSIBLE PARKING DETAILS
C504 SCALE: NONE

Wilbur D. Thesier, P.E. PC
PROFESSIONAL ENGINEER
PO BOX 870, CARTHAGE, NY 13619
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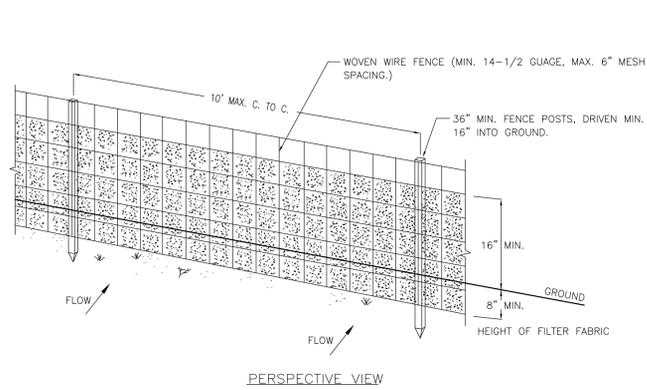
REVISIONS

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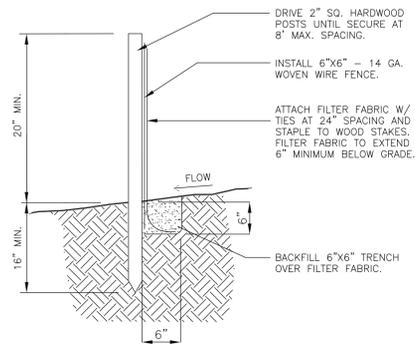
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SITE DETAILS



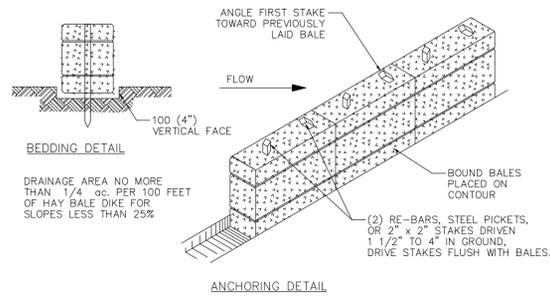
PERSPECTIVE VIEW

1 TYPICAL SILT FENCE DETAILS
 C508 SCALE: NONE



NOTES

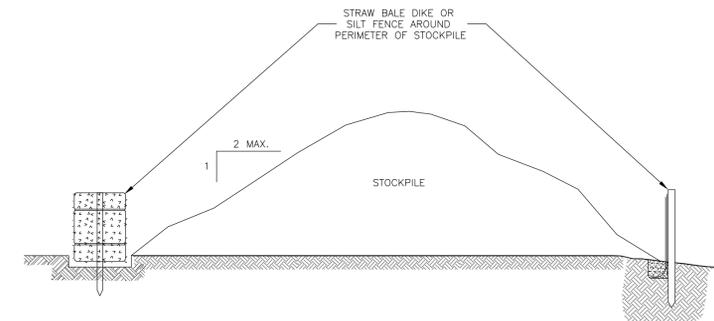
1. PRE-MANUFACTURED SILT FENCE TO BE FASTENED SECURELY TO FENCE POSTS WITH WIRE TIES OR STAPLES.
2. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER THEY SHALL BE OVERLAPPED BY 152 (6") AND FOLDED.
3. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN "BULGES" DEVELOP IN THE SILT FENCE.
4. SILT FENCE TO REMAIN IN PLACE UNTIL LAWNS HAVE BEEN ESTABLISHED AND/OR FINISH SURFACE HAVE BEEN INSTALLED.



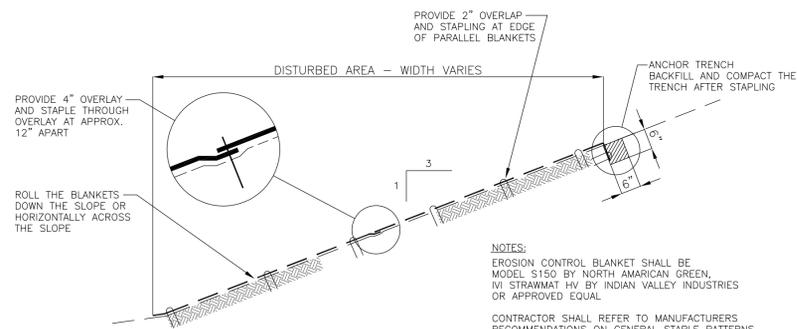
CONSTRUCTION SPECIFICATIONS

1. BALES SHALL BE PLACED AT THE TOE OF A SLOPE OR ON THE CONTOUR AND IN A ROW WITH ENDS TIGHTLY ABUTTING THE ADJACENT BALES.
2. EACH BALE SHALL BE EMBEDDED IN THE SOIL A MINIMUM OF 4" AND PLACED SO THE BINDINGS ARE HORIZONTAL.
3. BALES SHALL BE SECURELY ANCHORED IN PLACE BY EITHER TWO STAKES OR RE-BARS DRIVEN THROUGH THE BALE. THE FIRST STAKE IN EACH BALE SHALL BE DRIVEN TOWARD THE PREVIOUSLY LAID BALE AT AN ANGLE TO FORCE THE BALES TOGETHER. STAKES SHALL BE DRIVEN FLUSH WITH THE BALE.
4. INSPECTION SHALL BE FREQUENT AND REPAIR REPLACEMENT SHALL BE MADE PROMPTLY AS NEEDED.
5. BALES SHALL BE REMOVED WHEN THEY HAVE SERVED THEIR USEFULNESS SO AS NOT TO BLOCK OR IMPEDE STORM FLOW DRAINAGE.

2 TYPICAL STRAW BALE DIKE DETAIL
 C508 SCALE: NONE



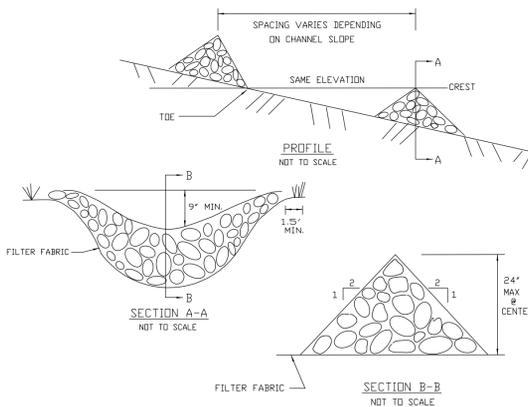
3 TYPICAL SOIL STOCKPILING DETAIL
 C508 SCALE: NONE



NOTES

- EROSION CONTROL BLANKET SHALL BE MODEL S150 BY NORTH AMERICAN GREEN, M STRAWMAT HV BY INDIAN VALLEY INDUSTRIES OR APPROVED EQUAL.
 CONTRACTOR SHALL REFER TO MANUFACTURERS RECOMMENDATIONS ON GENERAL STAPLE PATTERNS FOR SLOPE INSTALLATION.
 CONTRACTOR SHALL REFER TO MANUFACTURERS RECOMMENDATIONS FOR LAYING DIRECTION AND OVERLAP DIMENSIONS.

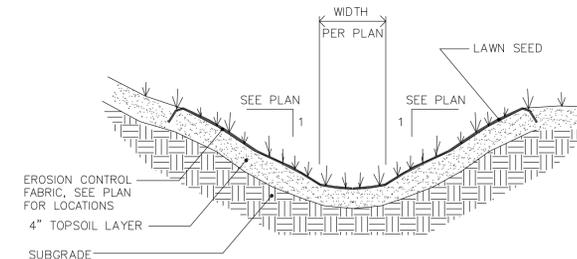
4 TYPICAL SIDESLOPE DETAIL - EROSION CONTROL BLANKET
 C508 SCALE: NONE



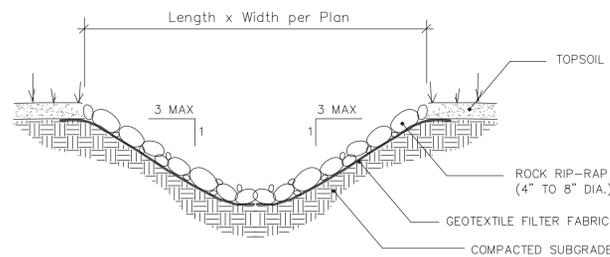
CONSTRUCTION SPECIFICATIONS

1. STONE WILL BE PLACED ON A FILTER FABRIC FOUNDATION TO THE LINES, GRADES AND LOCATIONS SHOWN IN THE PLAN.
2. SET SPACING OF CHECK DAMS TO ASSUME THAT THE ELEVATIONS OF THE CREST OF THE DOWNSTREAM DAM IS AT THE SAME ELEVATION OF THE TOE OF THE UPSTREAM DAM.
3. EXTEND THE STONE A MINIMUM OF 1.5 FEET BEYOND THE DITCH BANKS TO PREVENT CUTTING AROUND THE DAM.
4. PROTECT THE CHANNEL DOWNSTREAM OF THE LOWEST CHECK DAM FROM SCOUR AND EROSION WITH STONE OR LINER AS APPROPRIATE.
5. ENSURE THAT CHANNEL APPURTENANCES SUCH AS CULVERT ENTRANCES BELOW CHECK DAMS ARE NOT SUBJECT TO DAMAGE OR BLOCKAGE FROM DISPLACED STONES.

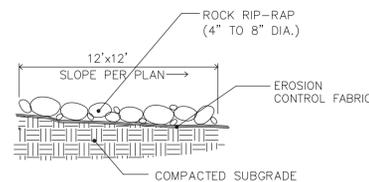
5 TYPICAL ROCK CHECK DAM DETAIL
 C508 SCALE: NONE



6 TYP. VEGATATED DRAINAGE CHANNEL
 C504 SCALE: NONE



7 TYP. RIP-RAP DRAINAGE CHANNEL
 C504 SCALE: NONE



8 RIP-RAP SLOPE DETAIL
 C504 SCALE: NONE

Wilbur D. Thesier, P.E. PC
PROFESSIONAL ENGINEER
 PO BOX 870, CARTHAGE, NY 13619
 PHONE: (315) 493-1966

New Expansion Project
Parkside Bible Church
 491 Eastern Boulevard, Huntington Street
 City of Watertown, Jefferson County, New York

REVISIONS

DATE	DESCRIPTION	BY
09/17/12	RIP-RAP SWALE DETAIL	WJL

DATE	08/20/12
DRAWN	W.J.L.
CHECKED	S.O.
SCALE	AS SHOWN

DRAWING NO.

C505